

Memorandum

BUILDING RELATIONSHIPS ONE PROJECT AT A TIME

To: Sunny Ghia, PE, QSD

Ghia Management Services, Inc.

25 E. Airway Blvd Livermore, CA 94551

From: Mario Tambellini, PE, TE

Pranesh Tarikere, PE

Date: July 28, 2023

Subject: Brentwood Popeyes Fast Food Restaurant Local Traffic Analysis (LTA)

INTRODUCTION

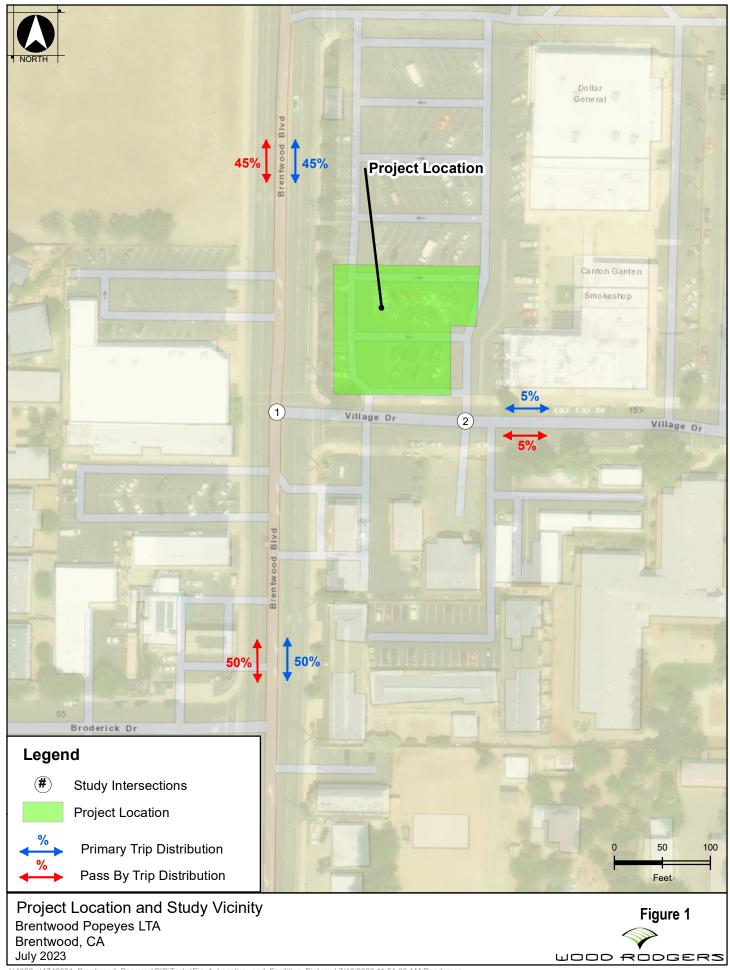
This Local Traffic Analysis (LTA) has been prepared to present the results of a traffic assessment for the proposed Popeyes Fast Food Restaurant Project (Project) located in the City of Brentwood (City). The Project would construct a Popeyes restaurant with drive-through on a site located on the existing southwest area of the Brentwood Plaza Shopping Center at the northwest corner of the intersection of Brentwood Boulevard & Village Drive. The Project location is shown in **Figure 1**. There is a currently a Dollar General and several small retail businesses located along the eastern portion of the Project site. The Project would demolish the existing parking located in the southwest corner of this retail center. This LTA includes the following:

- Project trip generation
- Intersection and roadway operations analysis
- Site access analysis, including Project driveway throat length
- Discussion of Project impact on existing multimodal facilities
- On-site parking evaluation
- Drive-through queueing analysis

This LTA has been prepared based on the 2014 City of Brentwood General Plan and the 2011 Brentwood Boulevard Specific Plan.

PROJECT DESCRIPTION

The Project proposes to construct a 2,583 square-foot Popeyes restaurant with a drive-through on a site that currently contains parking for various existing retail buildings. Project access would be provided via an existing driveway on Village Drive located approximately 200 feet east of the intersection of Brentwood Boulevard with Village Drive. There is one additional existing driveway on Brentwood Boulevard that serves the Brentwood Plaza Shopping Center to the north of the Project site, however, this analysis conservatively assumes all Project traffic would utilize the existing driveway on Village Drive. The 2014 City of Brentwood General Plan designates the site as part of the 2011 Brentwood Boulevard Specific Plan (BBSP), which accommodates a range of residential, commercial, office, mixed use, and other complementary uses that encourage the revitalization of the Brentwood Boulevard corridor within the Brentwood Boulevard Specific According to the BBSP's Zoning Map, the site is zoned Commercial/Office/Industrial/Residential (COIR). The Project site plan is included in **Attachment A**.



ANALYSIS SCENARIOS AND STUDY FACILITIES

Weekday Midday peak hour and Weekday PM peak hour Intersection operations were studied under the following scenarios:

- Existing Conditions
- Existing Plus Project Conditions

The following intersections and roadway facilities were included in this analysis:

Study Intersections:

- 1. Brentwood Boulevard & Village Drive
- 2. Brentwood Plaza Shopping Center Driveway & Village Drive

The study facilities are shown in **Figure 1**.

ANALYSIS METHODOLOGY

INTERSECTION OPERATIONS ANALYSIS

Synchro 11 software and Highway Capacity Manual, 6th Edition (HCM 6th Edition) methodology was used to determine intersection delay and LOS operations under Existing and Existing Plus Project weekday Midday and PM peak hour conditions.

For signalized intersections, the intersection delays and LOS reported are the average values for the whole intersection. For one-way stop-controlled (OWSC) intersections, the worst approach/movement delay and LOS is reported. The delay-based HCM 6th Edition LOS criteria for different types of intersection controls are outlined in **Table 1**.

Intersection Control Delay Level of (seconds/vehicle) **Description** Service Unsignalized **Signalized** Α Free-flow conditions with negligible to minimal delays. $delay \le 10.0$ $delay \le 10.0$ В Good progression with slight delays. $10.0 < delay \le 15.0$ $10.0 < delay \le 20.0$ \mathbf{C} $15.0 < delay \le 25.0$ $20.0 < \text{delay} \le 35.0$ Relatively higher delays. D Somewhat congested conditions with longer but tolerable delays. $25.0 < delay \le 35.0$ $35.0 < delay \le 55.0$ Е Congested conditions with significant delays. $35.0 < \text{delay} \le 50.0$ $55.0 < delay \le 80.0$ Jammed or grid-lock type operating conditions. delay > 50.0 delay > 80.0 Source: HCM 6th Edition Exhibit 19-8 and 20-2.

Table 1. HCM 6th Edition Intersection LOS Thresholds

LEVEL OF SERVICE CRITERIA

The City's General Plan Circulation Element Policy CIR 1-4 states that when a traffic analysis indicates that the Level of Service (LOS) for a street designated as a Route of Regional Significance should be maintained at "D" or better. Policy CIR 1-5 states that intersections that are unsignalized and not designated as Routes of Regional Significance shall be maintained at "D" or better, with controlled movements allowed at "E" or "F" if the intersection operates at LOS "C" overall, and/or if the "Peak Hour" signal warrant remains unmet. As both study intersections are unsignalized and Brentwood Boulevard is classified as a Route of Regional Significance, based on City general Plan requirements, the minimum acceptable LOS for the study intersections is considered to be LOS "D".

SIGNAL WARRANTS

California Manual on Uniform Traffic Control Devices (CA MUTCD) Peak Hour Signal Warrant #3 was used to evaluate the potential need for installation of a traffic signal at the intersection of Brentwood Boulevard & Village Drive.

OPERATIONS ANALYSIS

EXISTING TRAFFIC VOLUMES

Weekday Midday and PM peak hour turning movement counts were collected on Thursday, January 12, 2023 between 11:30 AM to 1:30 PM and between 4:00 PM to 6:00 PM. Traffic data count sheets are included in **Attachment B**. A summary of the lane geometry and intersection turning movements for Existing conditions are presented in **Figures 2** and **3**, respectively.

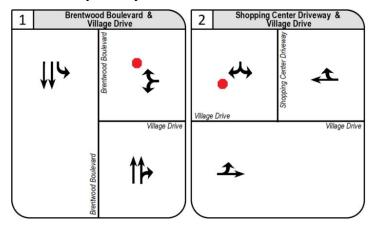


Figure 2. Existing Conditions Lane Geometrics and Control

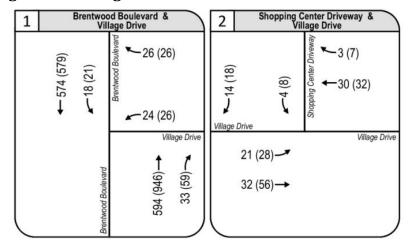


Figure 3. Existing Conditions Traffic Volumes

PROJECT TRIP GENERATION AND DISTRIBUTION

The trip generation data contained in the *Institute of Transportation Engineers (ITE) Trip Generation Manual,* 11th Edition, was used to estimate the number of trips generated by the Project. The ITE land use category of Fast-Food Restaurant with Drive Through (ITE Code 934) was used to represent the Project. **Table 2** shows the Project trip generation estimate.

Table 2. Project Trip Generation

Land Has	Overtites	IInita	Doile	Midd	ay Peak	Hour	PM	Peak Ho	our
Land Use	Quantity	Units	Daily	In	Out	Total	In	Out	Total
Fast Food (with drive-through) ¹	2.583	KSF ²	1,208	67	65	132	44	41	85
Pass-By Trips (Dail	y/Midday/PM	= 55%) ³	664	36	36	72	24	23	47
	Primar	y Trips ⁴	544	31	29	60	20	18	38

Notes:

¹The daily trip rate is based on the fitted curve equation and peak hour trip rates are based on the average rates for the proposed land use consistent with information contained in the ITE Trip Generation Manual, 11th Edition. Midday trip generation is based on weekday PM peak hour of generator rates.

As illustrated in Error! Reference source not found, the proposed Project is anticipated to generate a total of 544 daily primary trips, 60 Midday peak hour primary trips (31 inbound, 29 outbound), and 38 PM peak hour primary trips (20 inbound, 18 outbound) under typical weekday traffic demand conditions.

Project trips were assigned to the surrounding roadway network based on the following distribution, which was developed based on Project characteristics, existing travel patterns, and knowledge of the area. Primary Project trips and pass-by trips were assigned to the roadway network based on distributions shown in **Figure 1**. Pass-by trips are considered vehicle trips currently on the local roadway network that would make a short diversion to visit the project site. In this case, pass-by trips were considered to originate from vehicles traveling northbound or southbound on Brentwood Boulevard and eastbound and westbound on Village Drive.

Primary and pass-by distributions are illustrated in **Figure 1**. Primary Project Trip assignment is shown in **Figure 4** and Pass-by Project Trip assignment is shown in **Figure 5**. Primary project trips and pass-by trips are added to Existing volumes to obtain Existing Plus Project peak hour volumes, which are shown in **Figure 6**.

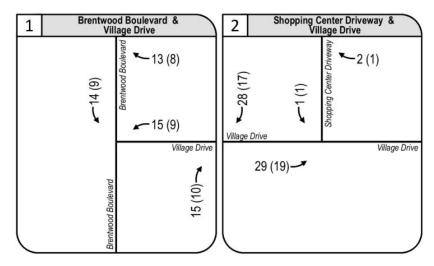


Figure 4. Primary Project Trip Assignment - Midday Peak Hour (PM Peak Hour)

²KSF = thousand square feet

³PM Pass-By percentages are based on data contained in the ITE Trip Generation Manual, 11th Edition Pass-By Tables Appendices.

⁴Primary Trips = Generated Trips - Pass-By Trips

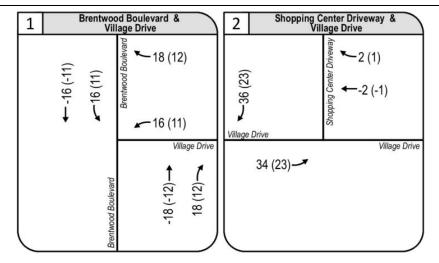


Figure 5. Pass-By Project Trip Assignment - Midday Peak Hour (PM Peak Hour)

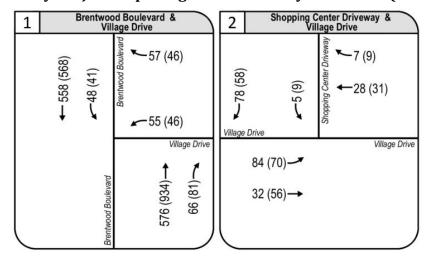


Figure 6. Existing Plus Project Conditions Traffic Volumes – Midday Peak Hour (PM Peak Hour)

INTERSECTION LEVEL OF SERVICE

Table 3 presents a summary of the intersection LOS operations under weekday Midday and PM peak hour Existing and Existing Plus Project conditions.

As shown in **Table 3**, all intersections operate at acceptable LOS (LOS "D" or better) under Existing and Existing Plus Project conditions. Synchro software HCM 6th Edition intersection LOS output reports are included in **Attachment C**.

CA MUTCD Peak Hour Signal Warrant #3 is currently unmet at the Brentwood Boulevard & Village Drive intersection. Signal warrant worksheets are provided in **Attachment D**.

Table 3. Intersection Operations

#	Intersection	Control	LOS	Peak	Existing	ţ	Wrnt	Existing Pl Project		Wrnt
#	Intersection	Туре	Criteria	Hour	Delay (sec/veh) ²	LOS	Met? ³	Delay (sec/veh) ²	LOS	Met?
1	Brentwood Boulevard	OWSC ¹	D	Midday	13.2	В	No	15.1	С	No
1	& Village Drive	OW3C ²	D	PM	19.8	С	No	23.9	С	No
	Brentwood Plaza Shopping Center			Midday	8.8	Α	-	9.1	Α	-
2	Driveway & Village Drive	OWSC ¹	D	PM	8.9	A	-	9.0	A	_

Notes:

INTERSECTION QUEUEING ANALYSIS

Vehicle queuing was analyzed at the study intersections for all stop-controlled movements and movements with turn pockets that the Project would add trips to. **Table 4** shows the available storage lengths and 95th percentile queues under all analysis scenarios. As shown in **Table 4**, all 95th percentile queues are anticipated to be accommodated by the existing available storage for all study scenarios.

Table 4. Queueing Analysis Results

			Available		95 th Percenti	le Queue (ft)
#	Intersection	Movement	Storage (ft) ¹	Peak Hour	Existing	Existing Plus Project
		SBL	80	Midday	<20	<20
1	Brentwood Boulevard & Village	SDL	80	PM	<20	<20
1	Drive	MD	115	Midday	<20	<20
		WB	115	PM	<20	30
2	Brentwood Plaza Shopping	CD		Midday	<20	<20
2	Center Driveway & Village Drive	SB		PM	<20	<20

Notes: One queued vehicle length is considered to be 20 feet long.

INTERNAL SITE CIRCULATION AND PARKING

BRENTWOOD PLAZA SHOPPING CENTER DRIVEWAY THROAT LENGTH

The throat length for vehicles exiting the Brentwood Plaza Shopping Center Driveway onto Village Drive is approximately 100 feet. The 95th percentile queues for the southbound approach of the Project driveway are projected to be one vehicle (approximately 20 feet) or less under Existing Plus Project conditions. Thus, the throat length is considered sufficient based on the 95th percentile queues shown in the Synchro output reports included in **Attachment C**.

TRANSIT, BICYCLES, AND PEDESTRIANS

Transit service in the City of Brentwood is provided by Tri Delta Transit (TDT). The nearest transit stops to the Project site are located within 200 feet of the Brentwood Boulevard & Village Drive intersection and are served by the 391, 300X and 384 Bus Routes, located approximately 200 and 300 feet from the site. The 391 Brentwood Park & Ride/Pittsburg Center Station, 300X Brentwood Park & Ride/Antioch BART, and 384 Brentwood Park & Ride/Antioch BART serves the Tri Delta Transit system. The Tri Delta Transit service is

¹ OWSC = One-Way Stop-Controlled

 $^{^{2}}$ For OWSC, the worst approach/movement delay and LOS is reported.

³ Wrnt Met? = Peak Hour Signal Warrant #3

¹ For stop-controlled movements, available storage represents the distance to the nearest cross-street.

accessible from the Project site via existing pedestrian facilities along Brentwood Boulevard. The Project is not anticipated to adversely affect existing transit facilities.

Existing striped bike lanes are present along Brentwood Boulevard north and south of Village Drive. Based on the Project site plan, adequate onsite bicycle parking and storage for patrons will be provided.

Existing sidewalks are present along both sides of Brentwood Boulevard and Village Drive within the Project vicinity, with existing curb ramps and pedestrian crossings present on two of the three legs of the Brentwood Boulevard & Village Drive intersection. The Project would provide connectivity to the existing sidewalk 40 feet north of the northeast corner of the Brentwood Boulevard & Village Drive intersection. The Project would provide adequate connectivity to existing bicycle and pedestrian facilities.

PARKING

Based on requirements outlined in Section 17.620.012 of the City Zoning Ordinance, the Project is required to provide 1 parking space per 100 square foot gross floor area (GFA), plus 1 space per 50 square feet of gross floor area used for other assembly uses. However, when located in shopping centers, the number of required parking spaces may be reduced at the time of issuance of a conditional use permit. The Project proposes to provide 15 total parking spaces, including two (2) accessible parking space, plus an additional 140 parking spaces exist in the surrounding Brentwood Plaza Shopping Center. As the Project is located within a larger shopping center, the existing shared parking spaces would likely accommodate Project customers.

DRIVE-THROUGH QUEUEING ANALYSIS

A drive-through queueing analysis was performed for the Project based on data collected at other Popeye restaurants within 15 miles of the Project site. Drive-through queueing data over five-minute intervals was collected on Tuesday, June 06, 2023, between 12 PM to 1 PM and 5 PM to 6 PM, at the following locations:

- Popeyes at 5019 Lone Tree Way, Antioch, CA
- Popeyes at 101 Carol Lane, Oakley, CA
- Popeyes at 1283 East Leland Road, Pittsburg, CA

Drive-through queueing data is included in **Attachment E**. A summary of the drive-through queue data is included in **Table 5**.

Table 5. Maximum Drive-Through Queueing at Near-by Popeyes Restaurants

Table 3. Maximum Dilve inio	agn Queueing at near b	y i opcycs ic	cstaurants
	Available Drive-Through	Max Observed	Queue (veh (ft))
Location	Storage (ft) ¹	Noon	PM
Popeyes at 5019 Lone Tree Way, Antioch	150	6 (120')	6 (120')
Popeyes at 101 Carol Lane, Oakley	150	5 (100')	6 (120')
Popeyes at 1283 East Leland Road, Pittsburg	115	8 (160')	7 (140')
Notes: 1 Available drive-through storage is measured fi	rom the drive-throuah entrance to	the order pick-up	window(s).

The Pittsburg location was shown to experience the highest maximum queue of 8 vehicles (approximately 160 feet) during the Midday peak hour.

Based on the maximum queue data collected at the near-by Popeyes locations, the Project is likely to experience a maximum drive-through queue of 8 vehicles. As shown in **Attachment A**, the Project site plan would provide total queue storage of approximately 370 feet, or room for approximately 18 vehicles, and would therefore accommodate the maximum projected drive-through queue of 8 vehicles. The Project site also includes dual order boards. Projected maximum queues would not block the Brentwood Plaza Shopping Center Driveway of the Popeyes parking lot area.

CONCLUSION

The proposed Project is anticipated to generate a total of 544 daily primary trips, 60 Midday peak hour primary trips (31 inbound, 29 outbound), and 38 PM peak hour primary trips (20 inbound, 18 outbound) under typical weekday traffic demand conditions.

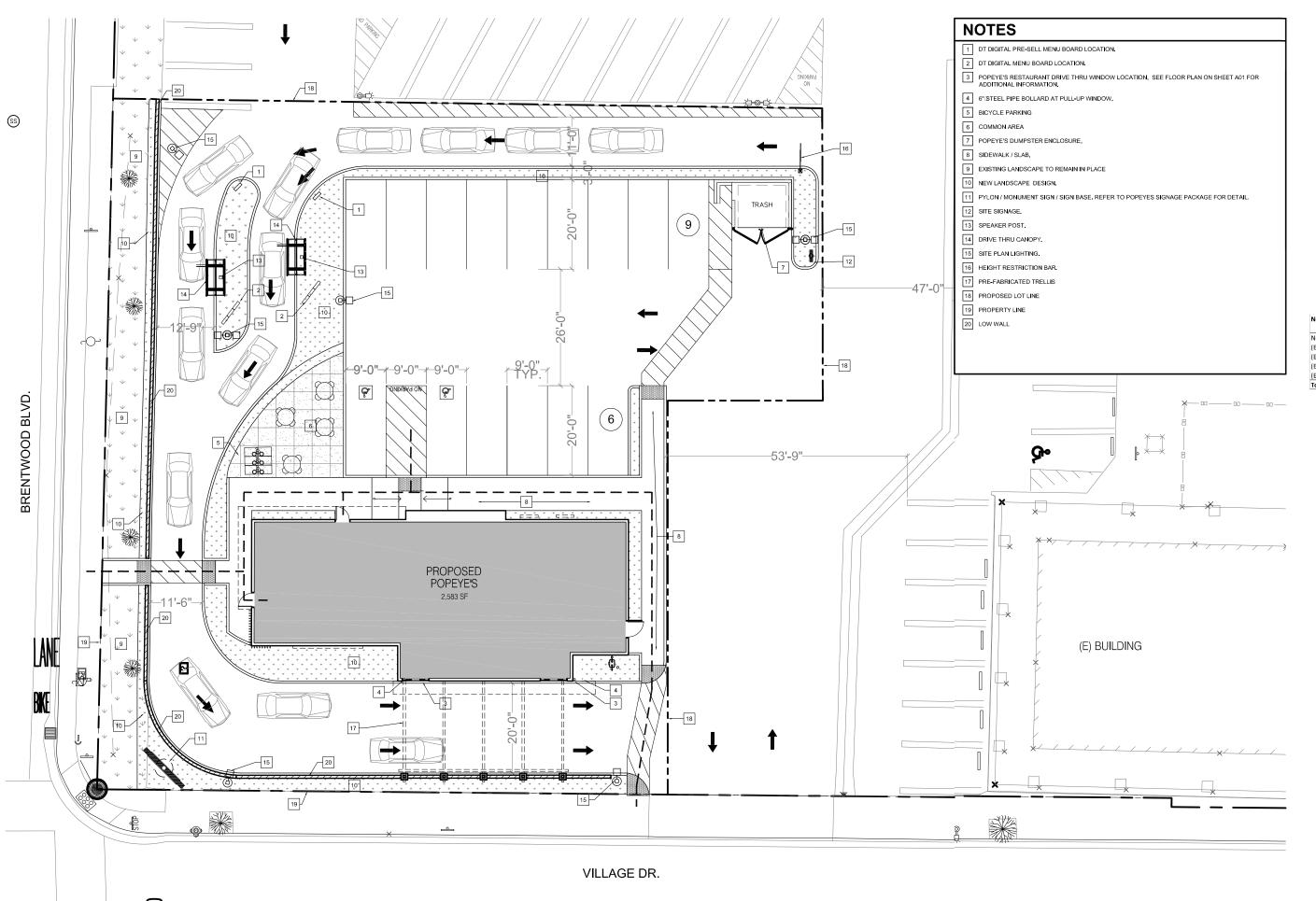
Intersection LOS at all study intersections was projected to be acceptable (LOS "D" or better) under all study scenarios. CA MUTCD Peak Hour Signal Warrant #3 is currently unmet at the Brentwood Boulevard & Village Drive intersection. All 95th percentile queues are anticipated to be accommodated by the existing available storage for all study scenarios.

The throat length for vehicles exiting the Project driveway onto Village Drive is approximately 100-feet. The 95th percentile queues for the southbound approach of the Project driveway are projected to be one vehicle (approximately 20 feet) or less under Existing Plus Project conditions. Thus, the driveway throat length is considered sufficient.

The Project would provide adequate connectivity to existing bicycle and pedestrian facilities. The Project proposes to provide 15 total parking spaces, including two (2) accessible parking spaces, with additional parking provided by existing spaces within the Brentwood Plaza Shopping Center.

A drive-through queueing analysis was performed for the Project based on data collected at similar drive-through restaurants within 15 miles of the Project vicinity, including Antioch, Oakley, and Pittsburg. Of the three sites, the Pittsburg location experience the maximum drive-through queue of 8 vehicles. The Project site has dual order boards and allow for a total drive-through queueing capacity of 18 vehicles. Based on the maximum queues collected at similar Popeyes sites, the Project is likely to experience a maximum drive-through queue of 8 vehicles. The Project site would accommodate the maximum projected drive-through queue.

ATTACHMENT A PROJECT SITE PLAN	





PROJECT MANAGER: MIKE JIBAJA 15998 SKYRIDGE DR RIVERSIDE, CA 92503 T: 909.520.3381 E: MIKE@AGJMDESIGN.COM

CLIENT: SUNNY GHAI GHAI MANAGEMENT SERVICES, INC. 25 E. AIRWAY BLVD. LIVERMORE CA T: 510.573.5905 E: SUNNY@GHAIMANAGEMENT.COM

New Popeys w/ Existing stripping New proposed stalls at popeyes (E) Accessible stalls (E) Center stalls (E) Permiter stalls (E) Stalls behind bldg

IMPROVEMENT AREA: 18,746 SF

TOTAL PARKING STALLS:

10 STANDARD STALLS 2 ACCESSIBLE STALLS

ADDRESS: BRENTWOOD BLVD. BRENTWOOD CA, 94513

ASSESSORS #: 016-150-106-9 ZONING: BBSP LOT SIZE: 3.4 ACRES

POPEYES

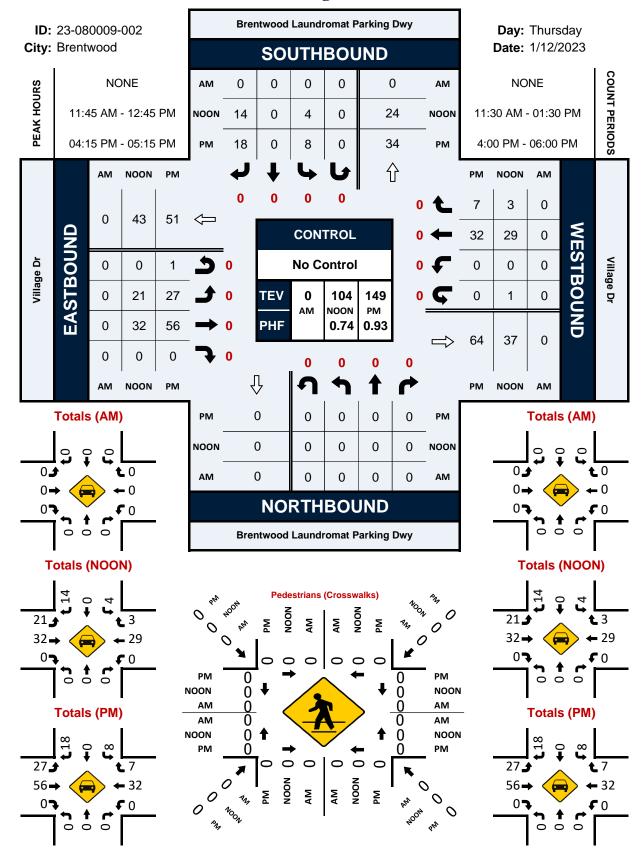
PROPOSED ENLARGED SITE PLAN

03.22.23

ATTACUMENT D	
ATTACHMENT B	
Traffic Counts	

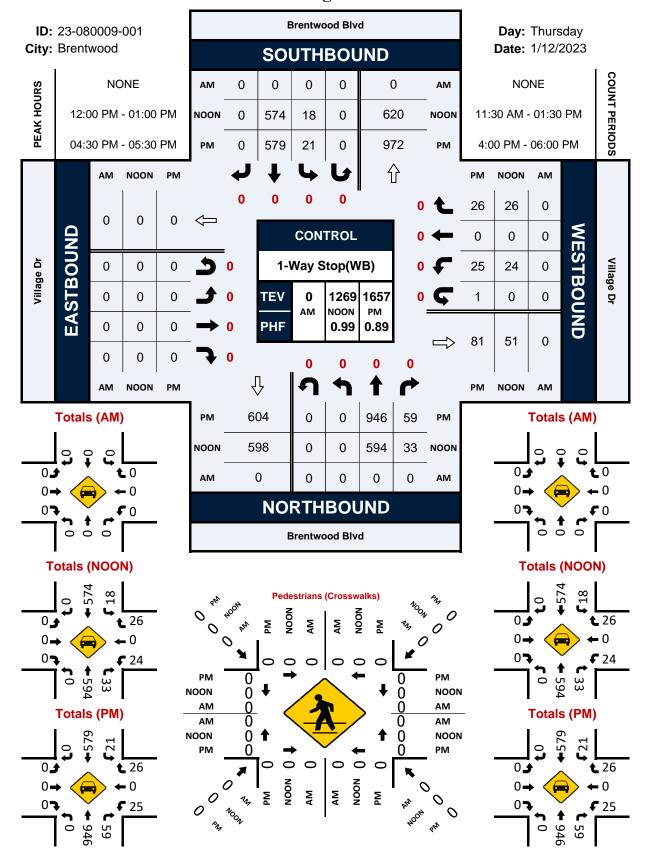
Brentwood Laundromat Parking Dwy & Village Dr

Peak Hour Turning Movement Count



Brentwood Blvd & Village Dr

Peak Hour Turning Movement Count



ATTACHMENT C
SYNCHRO HCM 6TH EDITION REPORTS

Intersection						
Int Delay, s/veh	0.7					
Movement		WBR	NDT	NDD	SBL	CDT
	WBL	WBR	NBT	NBR		SBT
Lane Configurations	7	00	↑	22	ነ	^
Traffic Vol, veh/h	25	26	594	33	18	574
Future Vol, veh/h	25	26	594	33	18	574
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	80	-
Veh in Median Storag	e,# 1	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	99	99	99	99	99	99
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	25	26	600	33	18	580
N.A. '. (N.A.)	N. 4					
	Minor1		/lajor1		Major2	
Conflicting Flow All	943	317	0	0	633	0
Stage 1	617	-	-	-	-	-
Stage 2	326	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	261	679	-	-	946	-
Stage 1	501	-	-	_	-	-
Stage 2	704	_	_	-	-	_
Platoon blocked, %	. o r		_	_		_
Mov Cap-1 Maneuver	256	679	_	_	946	_
Mov Cap-1 Maneuver		-	_	_	340	
Stage 1	501	-	-	<u>-</u>	-	-
	691		-	-		
Stage 2	091	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s			0		0.3	
HCM LOS	В					
Minor Lane/Major Mvr	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	488	946	-
HCM Lane V/C Ratio		-	-	0.106	0.019	-
HCM Control Delay (s)	-	-	13.2	8.9	-
HCM Lane LOS		-	-	В	Α	-
HCM 95th %tile Q(veh	1)	-	-	0.4	0.1	-
	,					

Intersection						
Int Delay, s/veh	3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		स	1>		W	
Traffic Vol, veh/h	21	32	30	3	4	14
Future Vol, veh/h	21	32	30	3	4	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-		-	
Storage Length	_	-	_	-	0	-
Veh in Median Storage		0	0	_	0	_
Grade, %	, <i>11</i>	0	0	_	0	_
Peak Hour Factor	74	74	74	74	74	74
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	28	43	41	4	5	19
IVIVIIIL FIOW	20	43	41	4	5	13
Major/Minor	Major1	N	Major2	ľ	Minor2	
Conflicting Flow All	45	0	-	0	142	43
Stage 1	-	-	-	-	43	-
Stage 2	-	-	-	-	99	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	_	-	5.42	_
Critical Hdwy Stg 2	-	_	-	-	5.42	-
Follow-up Hdwy	2.218	_	_	_	3.518	3.318
Pot Cap-1 Maneuver	1563	_	_	_	851	1027
Stage 1	-	_	_	_	979	-
Stage 2	_	_	_	-	925	_
Platoon blocked, %		_	_	_	320	
Mov Cap-1 Maneuver	1563	_	_	_	836	1027
Mov Cap-1 Maneuver	-	_	_	_	836	1021
Stage 1					961	
•	-	-	-	-	925	-
Stage 2	-	-	-	-	923	-
Approach	EB		WB		SB	
HCM Control Delay, s	2.9		0		8.8	
HCM LOS					Α	
Mineral and /Mineral	-4	EDI	CDT.	MOT	MPP	ODL 4
Minor Lane/Major Mvm	זנ	EBL	EBT	WBT	WBR	
Capacity (veh/h)		1563	-	-	-	977
HCM Lane V/C Ratio		0.018	-	-	-	0.025
HCM Control Delay (s)		7.3	0	-	-	8.8
HCM Lane LOS		Α	Α	-	-	Α
HCM 95th %tile Q(veh))	0.1	-	-	-	0.1

Intersection						
Int Delay, s/veh	0.8					
	WDI	WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		†		7	^
Traffic Vol, veh/h	25	26	946	59	21	579
Future Vol, veh/h	25	26	946	59	21	579
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	80	-
Veh in Median Storag	e, # 1	-	0	-	-	0
Grade, %	0	-	0	-	_	0
Peak Hour Factor	89	89	89	89	89	89
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	28	29	1063	66	24	651
IVIVIIIL I IOW	20	23	1003	00	24	051
Major/Minor	Minor1	N	Major1	N	Major2	
Conflicting Flow All	1470	565	0		1129	0
Stage 1	1096	-	-	-	-	_
Stage 2	374	<u>-</u>	<u>-</u>	_	_	_
Critical Hdwy	6.84	6.94	_	_	4.14	_
Critical Hdwy Stg 1	5.84	0.34		_	4.14	_
			-	-		
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	118	468	-	-	615	-
Stage 1	282	-	-	-	-	-
Stage 2	666	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	113	468	-	-	615	-
Mov Cap-2 Maneuver	220	-	_	-	-	-
Stage 1	282	-	_	-	_	-
Stage 2	640	_	_	_	_	_
Olago Z	U+U					
Approach	WB		NB		SB	
HCM Control Delay, s	19.8		0		0.4	
HCM LOS	С					
Minor Lane/Major Mvr	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	301	615	-
HCM Lane V/C Ratio		-	-	0.19	0.038	-
HCM Control Delay (s)	-	-	19.8	11.1	-
HCM Lane LOS		-	-	С	В	-
HCM 95th %tile Q(veh	1)	_	-	0.7	0.1	_
	.,			J.1	J. 1	

Intersection						
Int Delay, s/veh	2.9					
		FRT	WET	WEE	ODI	ODD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	1		W	
Traffic Vol, veh/h	28	56	32	7	8	18
Future Vol, veh/h	28	56	32	7	8	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	_	-	0	-
Veh in Median Storage	.# -	0	0	_	0	_
Grade, %	·, <i>'</i> ''	0	0	_	0	_
Peak Hour Factor	93	93	93	93	93	93
	2	2	2	2	2	2
Heavy Vehicles, %						
Mvmt Flow	30	60	34	8	9	19
Major/Minor	Major1	N	/lajor2		Minor2	
Conflicting Flow All	42	0	- najoiz	0	158	38
Stage 1	-	-	-	-	38	-
Stage 2	-	-	-	-	120	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1567	-	-	-	833	1034
Stage 1	-	-	-	-	984	-
Stage 2	-	-	-	-	905	-
Platoon blocked, %		_	_	_		
Mov Cap-1 Maneuver	1567	_	_	_	816	1034
Mov Cap-1 Maneuver	1001	_	_	_	816	-
	-	_			964	
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	905	-
Approach	EB		WB		SB	
HCM Control Delay, s	2.4		0		8.9	
	2.4		U			
HCM LOS					Α	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1567	_	_		955
HCM Lane V/C Ratio		0.019	_	-	_	0.029
HCM Control Delay (s)		7.3	0		_	8.9
					_	
HCM Lane LOS		A	Α	-	-	A
HCM 95th %tile Q(veh)		0.1	-	-	-	0.1

Intersection						
Int Delay, s/veh	1.5					
		WED	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		↑ ↑		<u>ነ</u>	^
Traffic Vol, veh/h	54	57	576	66	48	558
Future Vol, veh/h	54	57	576	66	48	558
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	80	-
Veh in Median Storag	e,# 1	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	99	99	99	99	99	99
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	55	58	582	67	48	564
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	994	325	0	0	649	0
Stage 1	616	-	-	-	-	-
Stage 2	378	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	_
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	242	671	_	_	933	_
Stage 1	501	-	_	_	-	_
Stage 2	663	_	_	_	_	_
Platoon blocked, %	000			<u>-</u>		_
Mov Cap-1 Maneuver	230	671	_	_	933	
Mov Cap-1 Maneuver						-
		-	-	-	-	
Stage 1	501	-	-	-	-	-
Stage 2	629	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s			0		0.7	
HCM LOS	C		- 0		0.1	
TIOWI LOG	U					
Minor Lane/Major Mvi	mt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	_	470	933	-
HCM Lane V/C Ratio		-	-	0.239		_
HCM Control Delay (s	3)	-	_	15	9.1	-
HCM Lane LOS		_	_	C	A	_
HCM 95th %tile Q(vel	h)	_	_	0.9	0.2	_
How John Johne Q(Ven	1)			0.0	0.2	

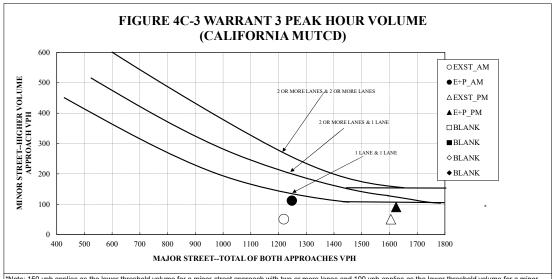
Intersection						
Int Delay, s/veh	5.9					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	4	₩ <u>₽</u>	WOR	ÿ#	אופט
Traffic Vol, veh/h	84	32	28	7	T 5	77
Future Vol, veh/h	84	32	28	7	5	77
Conflicting Peds, #/hr	0	0	0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	74	74	74	74	74	74
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	114	43	38	9	7	104
NA - ' / NA'	Maria		4.1.0		A' O	
	Major1		//ajor2		Minor2	
Conflicting Flow All	47	0	-	0	314	43
Stage 1	-	-	-	-	43	-
Stage 2	-	-	-	-	271	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	_	-	-	_	5.42	-
Follow-up Hdwy	2.218	-	_	_	3.518	3.318
Pot Cap-1 Maneuver	1560	_	_	_	679	1027
Stage 1	-	_	_	_	979	-
Stage 2	_	_	_	_	775	-
Platoon blocked, %		_	_	<u>-</u>	110	
	1560				628	1027
Mov Cap-1 Maneuver		-	-	-		
Mov Cap-2 Maneuver	-	-	-	-	628	-
Stage 1	-	-	-	-	906	-
Stage 2	-	-	-	-	775	-
Approach	EB		WB		SB	
HCM Control Delay, s	5.4		0		9.1	
HCM LOS	5.4		U		9.1 A	
HOWI LOS					А	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		1560	-	-	_	
HCM Lane V/C Ratio		0.073	_	_		0.112
HCM Control Delay (s)		7.5	0	_	_	9.1
HCM Lane LOS		7.5 A	A	_	_	9.1 A
HCM 95th %tile Q(veh	١	0.2	- -	-	-	0.4
HOW SOM MANE GIVEN)	U.Z	-	-	-	0.4

1: Brentwood Boulevard & Village Drive

Intersection						
Int Delay, s/veh	1.6					
		MDD	Not	NDD	051	057
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ħβ			^
Traffic Vol, veh/h	46	46	934	81	41	568
Future Vol, veh/h	46	46	934	81	41	568
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	80	-
Veh in Median Storage	e, # 1	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	89	89	89	89	89	89
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	52	52	1049	91	46	638
WINITE IOW	02	UL	1073	J	70	000
Major/Minor	Minor1	N	Major1	ا	Major2	
Conflicting Flow All	1506	570	0	0	1140	0
Stage 1	1095	-	-	-	-	-
Stage 2	411	-	-	_	_	_
Critical Hdwy	6.84	6.94	_	_	4.14	_
Critical Hdwy Stg 1	5.84	-	_	_	-	_
Critical Hdwy Stg 2	5.84	_	_	_	_	
Follow-up Hdwy	3.52	3.32	-	_	2.22	-
	112	465			609	
Pot Cap-1 Maneuver			-	-	009	-
Stage 1	282	-	-	-	-	-
Stage 2	638	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	103	465	-	-	609	-
Mov Cap-2 Maneuver	213	-	-	-	-	-
Stage 1	282	-	-	-	-	-
Stage 2	590	-	-	_	-	-
y -						
Approach	WB		NB		SB	
HCM Control Delay, s	23.9		0		8.0	
HCM LOS	С					
Minor Lone (Maior M	-4	NDT	NDD	MDL 4	CDI	CDT
Minor Lane/Major Mvn	π	NBT	NRK	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	292	609	-
HCM Lane V/C Ratio		-	-	0.354		-
HCM Control Delay (s)		-	-	23.9	11.4	-
HCM Lane LOS		-	-	С	В	-
HCM 95th %tile Q(veh)	-	-	1.5	0.2	-
J 222. 700.0 Q(1011	,					

1-4						
Intersection						
Int Delay, s/veh	4.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		सी	ĵ.		W	
Traffic Vol, veh/h	70	56	31	9	9	58
Future Vol, veh/h	70	56	31	9	9	58
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-			None	-	
Storage Length	_	-	_	-	0	-
Veh in Median Storag	e.# -	0	0	_	0	_
Grade, %	-	0	0	_	0	_
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	75	60	33	10	10	62
IVIVIIIL I IOW	13	00	00	10	10	02
Major/Minor	Major1	N	Major2		Minor2	
Conflicting Flow All	43	0	-	0	248	38
Stage 1	-	-	-	-	38	-
Stage 2	-	-	-	-	210	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	_	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1566	-	_	-	740	1034
Stage 1	-	-	_	-	984	-
Stage 2	_	_	_	_	825	_
Platoon blocked, %		<u>-</u>	_	_	320	
Mov Cap-1 Maneuver	1566	_	_	_	703	1034
Mov Cap-1 Maneuver		-		_	703	1034
Stage 1		_	_	_	935	_
	-		_	-	825	-
Stage 2	-	-	-	-	020	-
Approach	EB		WB		SB	
			WB 0		SB 9	
Approach HCM Control Delay, s HCM LOS						
HCM Control Delay, s					9	
HCM Control Delay, s HCM LOS	4.1	- FDI	0	MDT	9 A	ODI 4
HCM Control Delay, s HCM LOS Minor Lane/Major Mvi	4.1	EBL	0 EBT	WBT	9 A WBR	
HCM Control Delay, s HCM LOS Minor Lane/Major Mvi Capacity (veh/h)	4.1	1566	0 EBT	-	9 A WBR	972
HCM Control Delay, s HCM LOS Minor Lane/Major Mvi Capacity (veh/h) HCM Lane V/C Ratio	4.1 mt	1566 0.048	0 <u>EBT</u> -	WBT - -	9 A WBR	972 0.074
HCM Control Delay, s HCM LOS Minor Lane/Major Mvi Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s	4.1 mt	1566 0.048 7.4	0 EBT - - 0	-	9 A WBR	972 0.074 9
HCM Control Delay, s HCM LOS Minor Lane/Major Mvi Capacity (veh/h) HCM Lane V/C Ratio	mt (3)	1566 0.048	0 <u>EBT</u> -	-	9 A WBR	972 0.074

ATTACHMENT D CA MUTCD PEAK HOUR SIGNAL WARRANT #3	



*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

CCENADIO	APPROA	WARRANT	
SCENARIO -	MAJOR	MINOR	MET?
EXST_AM	1219	51	NO
E+P_AM	1248	112	NO
EXST_PM	1605	51	NO
E+P_PM	1624	91	NO
BLANK	0	0	NO
BLANK	0	0	NO
BLANK	0	0	NO
BLANK	0	0	NO

the highest of both approaches.

Date: July 13, 2023 Intersection No.: 1

Intersection: Brentwood Blvd & Village Dr

Number of lanes on MAJOR street: 2

Number of lanes on MINOR street: 1



ATTACHMENT E	
DRIVE-THROUGH QUEUEING DATA	

MAX QUEUE

Location: Popeyes Louisiana Kitchen, 5019 Lone Tree Wy

City: Antioch, CA

Date: 6/6/2023 (Tuesday)

MAX Queue Length (# of vehicles)					
TIME	Drive Thru Queue From Pickup Window to End of Queue	Notes			
12:00 PM	4				
12:05 PM	4				
12:10 PM	5				
12:15 PM	6				
12:20 PM	4				
12:25 PM	3				
12:30 PM	3				
12:35 PM	2				
12:40 PM	6				
12:45 PM	5				
12:50 PM	4				
12:55 PM	5				
5:00 PM	2				
5:05 PM	2				
5:10 PM	2				
5:15 PM	2				
5:20 PM	2				
5:25 PM	2				
5:30 PM	6				
5:35 PM	6				
5:40 PM	5				
5:45 PM	4				
5:50 PM	3				
5:55 PM	2				
Totals	89				

MAX QUEUE

Location: Popeyes Louisiana Kitchen, 101 Carol Ln

City: Oakley, CA

Date: 6/6/2023 (Tuesday)

MAX Queue Length (# of vehicles)					
TIME	Drive Thru Queue From Pickup Window to End of Queue	Notes			
12:00 PM	4				
12:05 PM	2				
12:10 PM	5				
12:15 PM	4				
12:20 PM	3				
12:25 PM	3				
12:30 PM	2				
12:35 PM	2				
12:40 PM	2				
12:45 PM	2				
12:50 PM	2				
12:55 PM	3				
5:00 PM	2				
5:05 PM	4				
5:10 PM	3				
5:15 PM	1				
5:20 PM	1				
5:25 PM	0				
5:30 PM	2				
5:35 PM	3				
5:40 PM	4				
5:45 PM	6				
5:50 PM	4				
5:55 PM	5				
Totals	69				

MAX QUEUE

Location: Popeyes Louisiana Kitchen, 1283 E Leland Rd

City: Pittsburg, CA
Date: 6/6/2023 (Tuesday)

MAX Queue Length (# of vehicles)					
TIME	Drive Thru Queue From Pickup Window to End of Queue	Notes			
12:00 PM	5				
12:05 PM	3				
12:10 PM	7				
12:15 PM	8				
12:20 PM	8				
12:25 PM	7				
12:30 PM	6				
12:35 PM	6				
12:40 PM	4				
12:45 PM	3				
12:50 PM	2				
12:55 PM	6				
5:00 PM	4				
5:05 PM	5				
5:10 PM	5				
5:15 PM	7				
5:20 PM	6				
5:25 PM	5				
5:30 PM	5				
5:35 PM	3				
5:40 PM	3				
5:45 PM	4				
5:50 PM	4				
5:55 PM	3				
Totals	119				